Comparative Study of Time-Domain versus Frequency-Domain Seismic Soil-Structure Interaction Analysis of Pressurized Water Reactor Containment Building

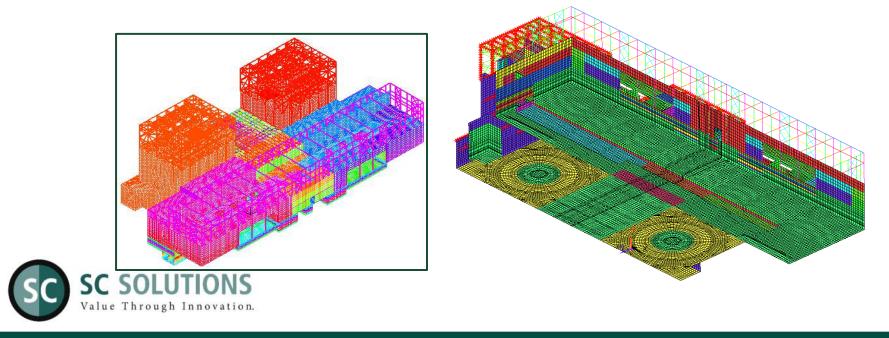
#### Payman Khalili Tehrani (Presenter) Benjamin Kosbab

October 19, 2016



## State of Practice: Nuclear Industry

- Pioneering industry in recognizing the importance of SSI
- Standard analysis approach: Equivalent Linear in Frequency Domain (ELFD) – Ground breaking in '80s, still perfectly fine for small-to-medium intensity shaking
- Historically limited to simplified (e.g. stick) models but detailed models possible today



## **ELFD Approach Limitations**

#### • Equivalent Linear Assumption

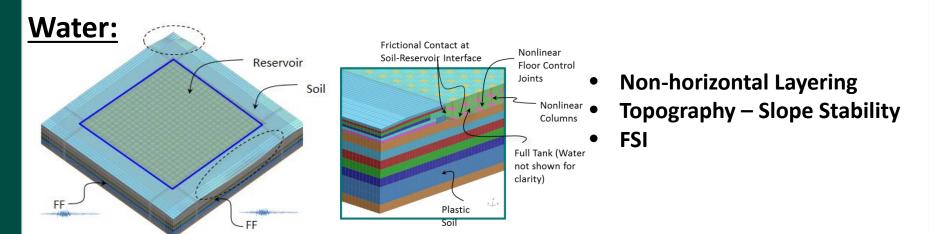
- <u>Linear</u> material properties for soil and structure
- <u>Tied</u> soil-structure interface behavior
- Applicable to small range of seismic hazard: (i.e. unique model needed for each hazard level)
- Cannot address seismic isolation, impact, etc.

#### Analysis Time

 Function of interaction nodes - Inefficient specially when dealing with deeply embedded structures

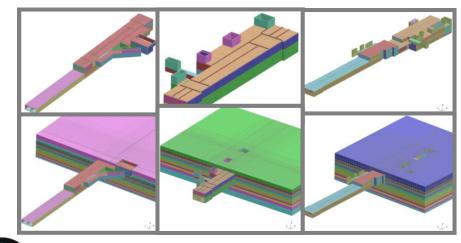


# Alternative: Nonlinear Time Domain (NLTD)



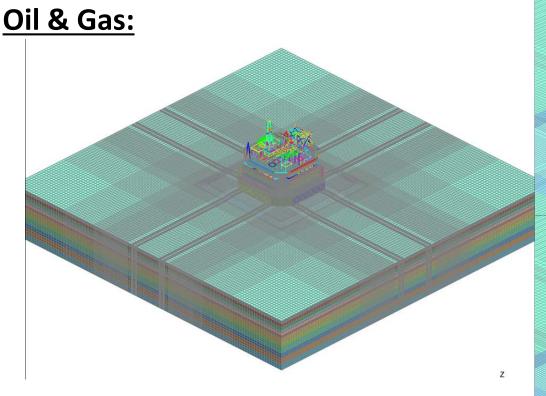
#### **Transportation:**

SC SOLUTIONS Value Through Innovation.



• Extreme shearing of soft soil layers due to seismic wave propagation

# Alternative: Nonlinear Time Domain (NLTD)



- Seismically isolated deck via LRBs
- **Unique Sandcore challenge**

Labs

Recent initiatives in nuclear industry for NLTD: ASCE4-16 and studies at National

#### Study Plan – Contribute to Industry Advancement

<u>Key:</u> Step by step confidence building, evolve from ELFD to NLTD: Two different approaches and different results not convincing

- Under similar and realistic assumptions, demonstrate a good match between TD and FD: Need a successful and consistent EL analysis in Time Domain (ELTD)
- Subsequently demonstrate potential savings/benefits offered by NLTD
- Demonstration via a realistic problem: detailed 3D FEM (not stick model), excited in all 3 directions, nuclear site and GM characteristics



## Equivalent Linear Time Domain (ELTD)

#### Challenges in TD:

Solution:

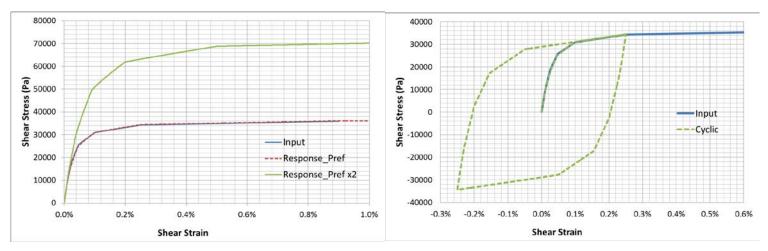
Solution:

- Soil domain truncation:
  - Truncation via PML
    - Radiation damping in a large-enough domain
- Frequency-Independent damping:
  - Abandon Historically simplified treatment of damping in TD, i.e. Rayleigh and modified Rayleigh
  - Achieve nearly hysteretic damping through Viscoelasticity

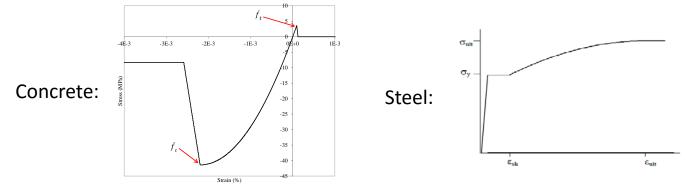


## Nonlinear Time Domain (NLTD)

• Soil plasticity with explicit hysteretic damping



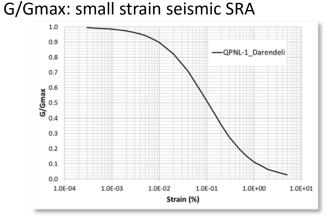
• Structural Material and geometric nonlinearities



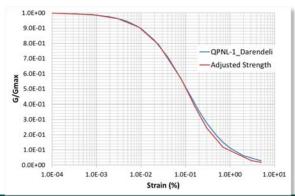
- Gapping and sliding at the soil-structure interface
- Base Isolation, FSI, etc.

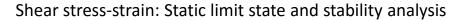
#### Soil Modeling: Improving the State of Practice

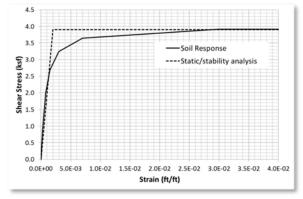
- Disconnect between small-strain and large strain response of the soil in geotechnical engineering practice
- Marriage between the two is necessary for large seismic events. (Stewart et al 2008)



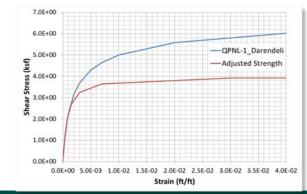
Hybrid G/Gmax: Seismic analyses







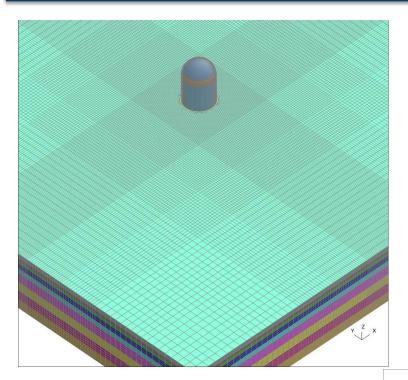
Hybrid Stress-Strain: Seismic analyses

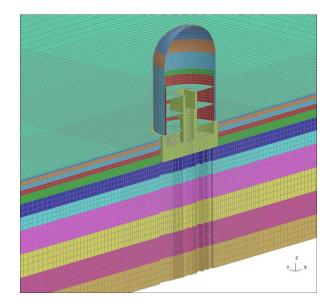


# PWR CONTAINMENT BUILDING ANALYSIS



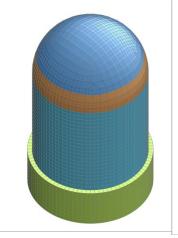
## FEM – PWR Containment Bldg

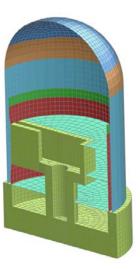




#### Structure: Elastic in all analyses

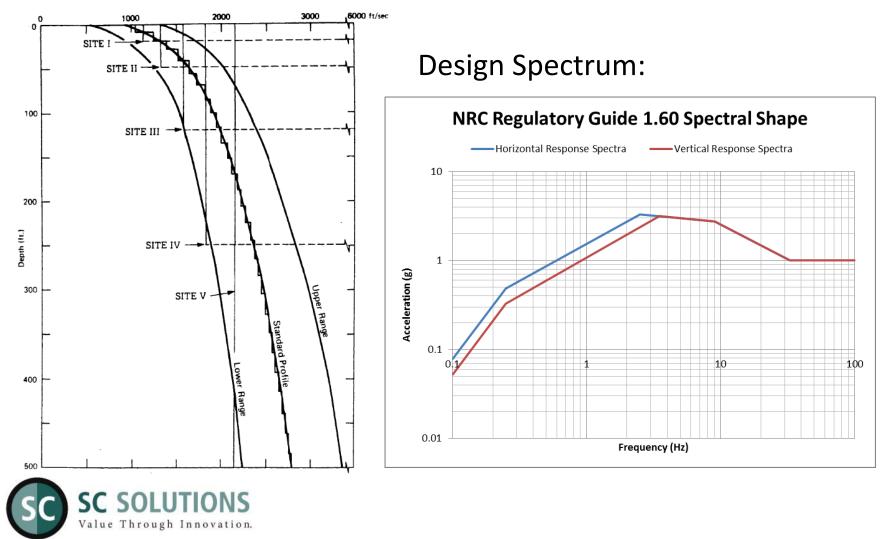




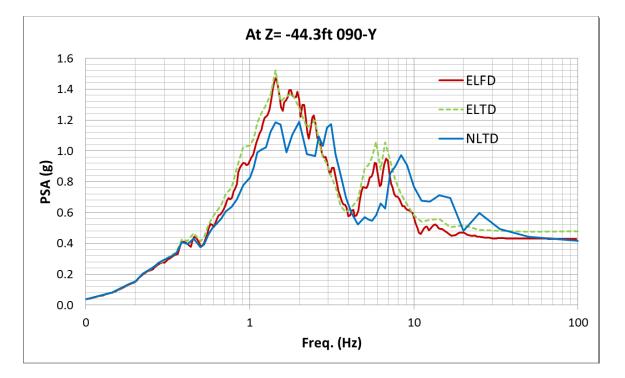


## Standard Nuclear Site and Spectrum

#### NUREG-CR-6865 Standard Nuclear Site IV



#### Site Response Verification – Different Approaches



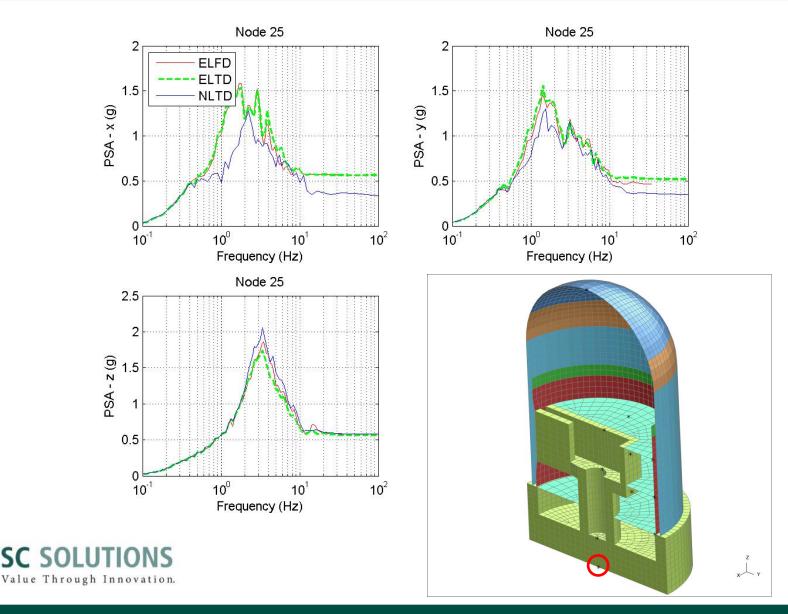
Soil hysteretic damping beyond 20%



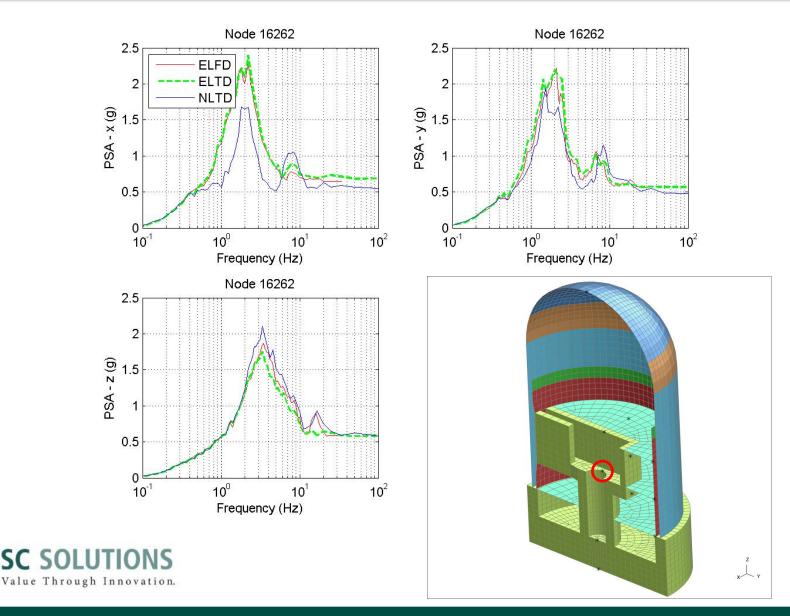
## RESULTS



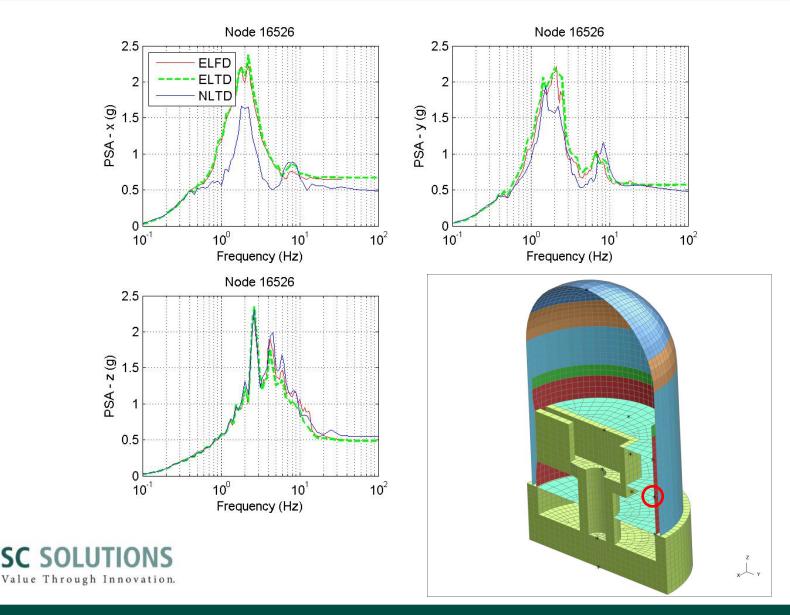
#### **Response: Below Reactor**



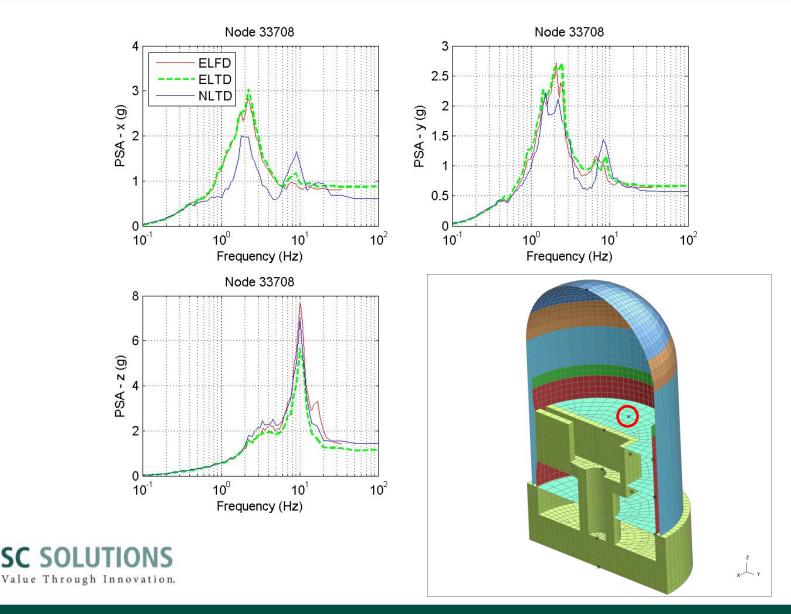
### Response: Above Core



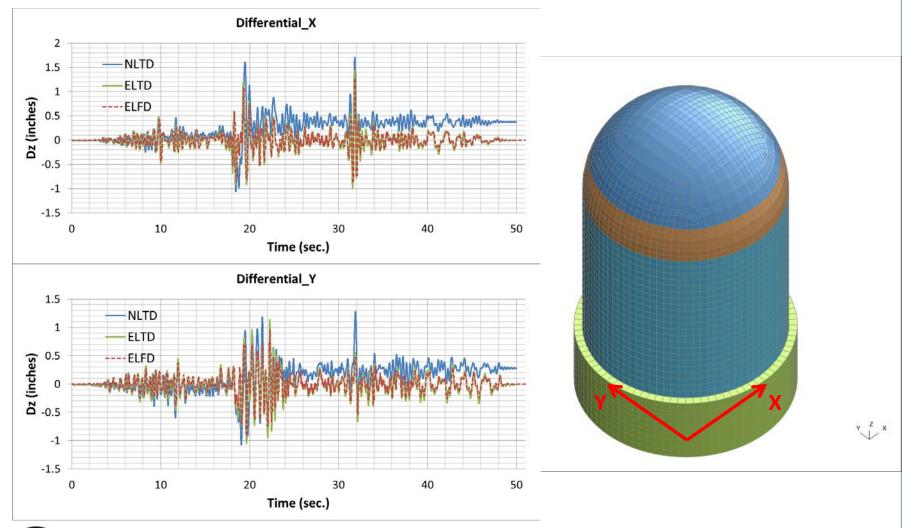
#### Response: Inner Wall Mid-Floor



#### Response: Mid-Slab Top Floor

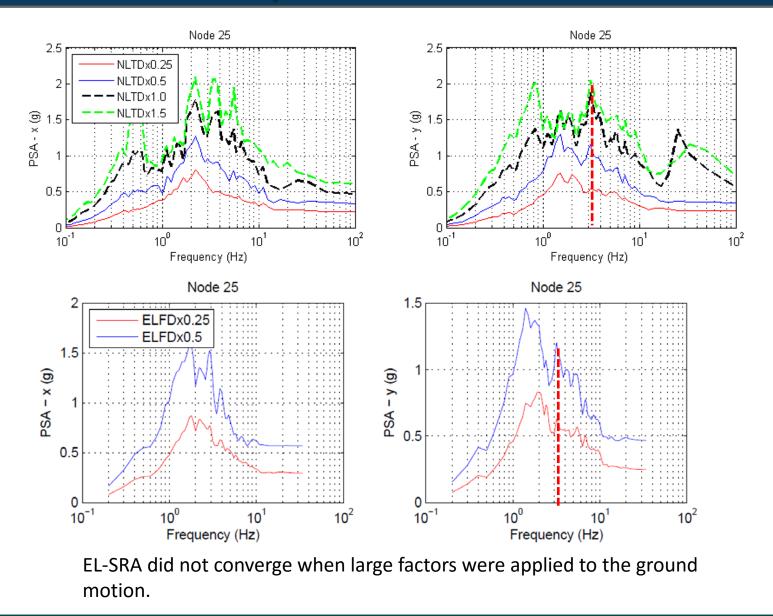


## **Response: Foundation Rocking**

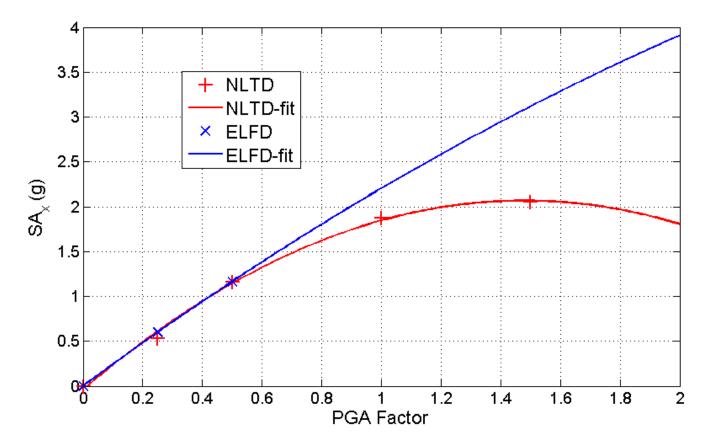




#### Scaled Response: Below Reactor



## Scaled Response: Below Reactor





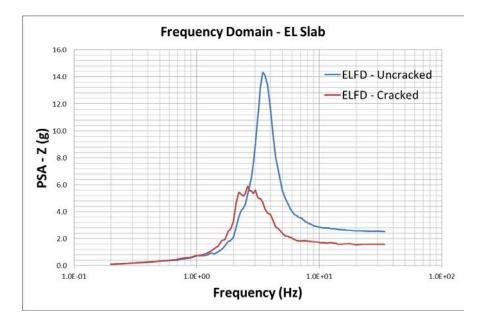
# EFFECT OF SLAB CRACKING ON VERTICAL RESPONSE



## Slab Cracking and Vertical Response - FD

Equivalent linear assumptions for the cracked section in FD (ASCE 4):

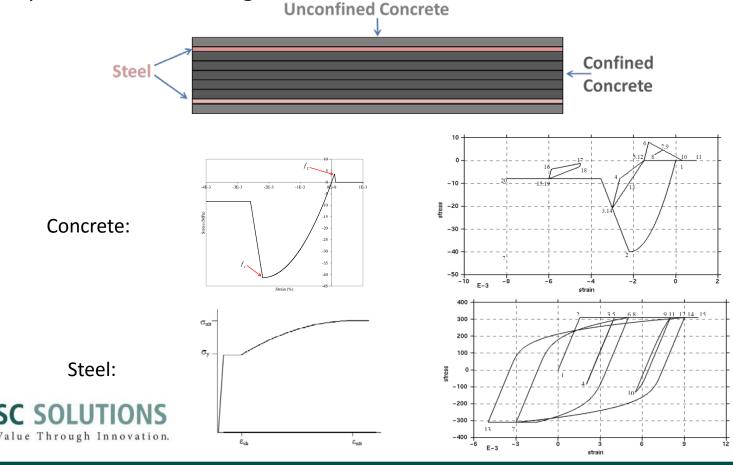
- 50% Cracked Section
- Damping increase from 4% to 7%



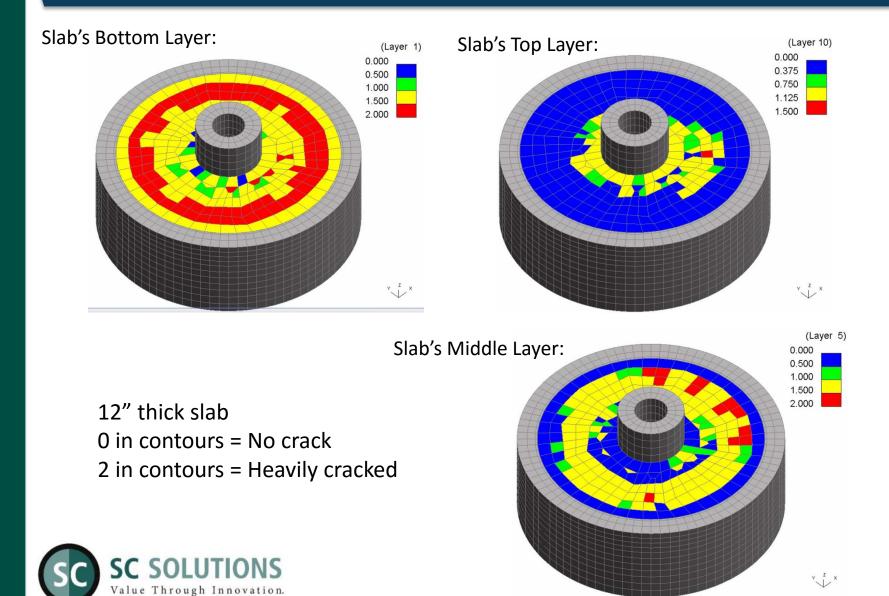


#### Modeling Nonlinear Response of RC Slab

- Layered composite shell finite elements
- Mander or Kent-Park model used for concrete layers/fibers
- concrete model : Cracking in tension, Crushing under compression, and post-peak strain softening.



#### Modeling Nonlinear Response of RC Slab



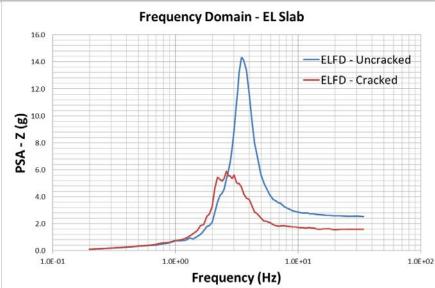
## Slab Cracking and Vertical Response

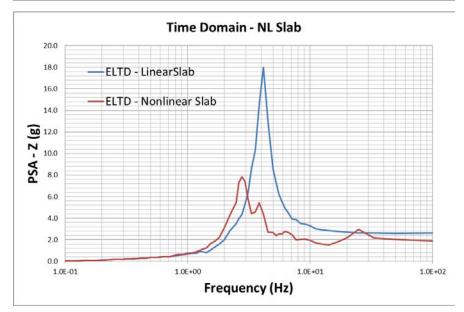
Equivalent linear assumptions in FD (ASCE 4):

- 50% Cracked Section
- Damping increase 4% to 7%

Observation from NL slab in TD:

- Period shift corresponding to 50% stiffness reduction
- Similar peak accelerations ratio







# **SUMMARY / CONCLUSIONS**

## $\mathsf{ELFD} \xrightarrow{\rightarrow} \mathsf{ELTD} \xrightarrow{\rightarrow} \mathsf{NLTD}$

- SSI is a key component to seismic evaluation of nuclear facilities and other critical infrastructure
- ELFD has been the long-accepted state-of-practice and has evolved to efficiently handle large and complex SSI problems
- TD approaches provide attractive alternatives to FD →
  Risk analyses and beyond-design-basis evaluations necessitate realistic response evaluations under large and varied seismic events
- ELTD with frequency-independent damping produces equivalent response as ELFD, thus verifying TD as legitimate tool
- NLTD analysis demonstrates ELFD can over-predict response for large intensity ground motions
- NLTD is versatile and can efficiently incorporate a variety of nonlinear response features in projects across multiple industries
- Selection of approach should be based on the applicability of the inherent technical assumptions, rather than limitations of tools and precedence

## **THANK YOU**

